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Design of a Technical-Artisanal Dike for Surface Water Storage and Artificial Recharge of the Manglaralto Coastal Aquifer. Santa Elena Parish, Ecuador



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ABSTRACT

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Manglaralto parish communities are supplied with drinking water extracted from shallow aquifers (associated with alluvial terraces) recharged by seasonal rivers. The companies responsible for extracting and distributing water are the Manglaralto Regional Drinking Water Administration Board (JAAPMAN, for its acronym in Spanish) and Olon Regional Board of Drinking Water (JRAPO, for its acronym in Spanish). However, due to population growth and tourism (floating population) of Manglaralto parish, water demand has increased. This has meant that, during the dry season, the water stored in these underground reservoirs is not enough to meet the growing demand. The case study presented is that of the Manglaralto community. The aim is to design, in a technical way, an artisanal dyke (tape) by performing geological-geotechnical studies that allow optimal surface storage and artificial recharge of aquifers for the provision of water to communities. The methodology is as follows: i) Analysis of the technical starting information, ii) Recognition of the study site and the choice of sampling sites, iii) Conducting laboratory tests and analysis of information obtained in the field, and iv) Technical design of the tape. According to laboratory results and the field data analysis, the design of a spillway dyke equipped with a dentellon was defined, which acts as a screen to block the subsurface flow. The rescue of ancestral knowledge, used as a solution to current problems in this coastal community, allows storing a volume of 4,641.88 m³ and an artificial recharge of 15%, with low costs and great social acceptance.

1. INTRODUCTION

Water is an essential element for a country's sustainable development as it plays an essential role in economic, social and human development [1, 2]. After thousands of years of human development in which water has been an abundant resource in most areas, the situation is now changing abruptly. Particularly in arid regions of the world, water scarcity has become the biggest threat to food security, human health and natural ecosystems [3, 4].

In order to achieve accessibility of water, the United Nations has proposed goals to achieve this objective, among which is "Significantly increase the efficient use of water resources in all sectors and ensure the sustainability of freshwater extraction and supply to address water scarcity and significantly reduce the number of people suffering from water shortages" [5]. To achieve water supply in some towns worldwide, people have used the extraction of water from

aquifers [6]. According to the International Atomic Energy Agency (IAEA), underground water is the largest drinking water source for humans. This represents 30.1% of the total fresh water in the world [7, 8].

The availability of freshwater resources is a challenge for many communities, becoming a scarce good that limits social and economic development [9-11]. Ironically, low households in developing countries spend a higher proportion of their income on water than in industrialised countries [12]. The outlook on the supply and renewal of water in the world is every day worse. This is because freshwater is not distributed equitably globally, nor the same quantity in the seasons or from one year to another [2, 13]. While the scarcity of freshwater resources already limits development and social well-being in many countries [10, 14-16], expected world population growth in the coming decades, coupled with growing economic prosperity, will increase water demand and exacerbate these problems [17, 18]. Therefore, it is essential to consider an integrality in water management to counteract the its lack [19, 20]. The two methods, proposed by Sixt et al. [19], Chica and Reyes [20], to manage water resources are increased supply (e.g. water storage by dike/dams, artificial recharge of aquifers, desalination of seawater or seeding and harvesting of water), and reduction of losses and waste (e.g. control of evaporation, more efficient industrial processes in the use of water, control of losses and waste in large cities).

In Ecuador, based on the National Plan for Good Living 2013-2017 (PNBV, for its acronym in Spanish), it was monitored that "83% of households have access to water through the public network" (goal 3.11.) [21]. The provinces with the highest water coverage are Pichincha, Santa Elena and El Oro. Santa Elena's province has 90.3% coverage by the public network [22, 23]. However, certain towns north of Santa Elena, such as Manglaralto and Olon, do not benefit from this service [4, 24]. Due to this, they have created water management boards (such as JAAPMAN and JRAPO), which have managed to supply the parish population through aquifers' exploitation. More than 30,000 people are supplied with this vital liquid by extracting water from wells located around the aquifers [25, 26].

However, the supply through aquifers also has its limitations, since population growth, lack of rain and increased tourism has generated a considerable water deficit. This leads to a shortage of water for the parish because the aquifer does not recharge quickly enough in the rainy season [20, 27]. Anciently, artisanal dikes (tapes) have been made in rivers around the world. Such is the case of the Miraflores district in Peru (built during pre-Inca times), where they have helped retain water, thus causing more significant infiltration of water into the aquifers and thus favouring their recharge [28, 29].

Among the strategies to try to solve the water shortage

during the dry seasons in the community of Manglaralto is the construction of tapes, which help retain water in various sectors of the river, thus contributing to the recharge of the aquifer [26]. However, the artisanal construction of these tapes is not durable since, according to the water capacity in the river's flood, it causes their collapse, having been used in their sound stage.

Given the conditions and the construction of artisanal tapes, built through community work and materials from the river, the community considered the construction of a technicalartisan tape, that is, to implement its construction with reinforced concrete. With this, the question arises: Is it possible that the design of a technical-artisan dike, built with the same ancestral characteristics but using concrete, is viable and allows the accumulation of water in it, helping to infiltrate a more significant amount of water into the aquifer and therefore solve the water needs of the Manglaralto population?

That is why the following objective is proposed: to design an artisanal dike (tape) in a technical way by carrying out topographic and geotechnical studies and laboratory tests, which allow optimal surface storage and artificial recharge the aquifers for the water provision to the Maglaralto community.

2. STUDY AREA

The study area in which this research is carried out is in the Manglaralto commune located in the same name's parish. Located northwest of the Santa Elena province, in the coastal region of Ecuador [30, 31]. The Manglaralto parish has an area of 497 km², bordered to the north and east by the province of Manabí, south by the Colonche parish and the west by the Pacific Ocean shown in Figure 1.

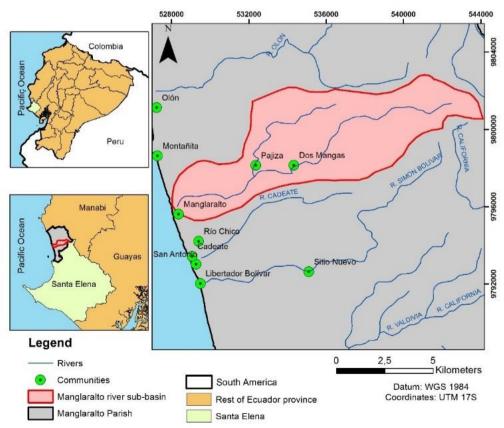


Figure 1. Parish Manglaralto community location. (Source: [32, 33])

The parish is located in the Manglaralto and Valdivia rivers' basins, with a basin head located in the hills of San Martin, La Culebra and La Cascada. The mountainous part of the parish is formed by the hills of Los Araujos, El Gallo, Culebra, San Martin, Colonche Los Lobos and the Olón Mountains with elevations ranging from 300 to 750 meters [26, 34].

The Manglaralto, Cadeate and Simón Bolívar rivers in the Manglaralto basin have recent alluvial deposits formed mainly of gravel and sand. The gravels present a variable classification along with the thickness of the layers and, also, they have a sandy matrix which makes them ideal for forming an aquifer. The geological formations that make up the Manglaralto River sub-basin are sedimentary and volcanosedimentary rocks with alluvial deposits found in flat reliefs bordered by elevations that exceed 100 meters in height [35-37].

The Manglaralto aquifer has a surface area of approximately 5.08 km² and the Manglaralto river basin, which feeds it, is 132.38 km² long. Manglaralto's climate is semi-arid, with an average annual temperature in 2017 of 27.3°C and an approximate rainfall of 256.1 mm and evapotranspiration of almost 1000 mm [38]. In the parish, JAAPMAN has 12 active

wells for the supply of 7 communes in the parish of Manglaralto: Montañita, Nueva Montañita, Manglaralto, Río Chico, Cadeate, San Antonio and Libertador Bolívar.

3. METHODOLOGY

The methodology used for the development of this research consisted of three stages, detailed in Figure 2.

3.1 Recognition of the study site

During the visit to the study site, which involved the riverbank, potential sites for constructing a dike (tape) were observed. These were selected for their characteristics, such as their geometry, terrain morphology, possible reservoir area, and the existence or not of an aquifer, according to studies by [26, 36]. They were identified and located near water wells, the reservoir area, neck, and sectors where the river's drainage contributes more water. At the end of this, the most suitable site for the design and construction of the technical tape (reinforced concrete) was selected.

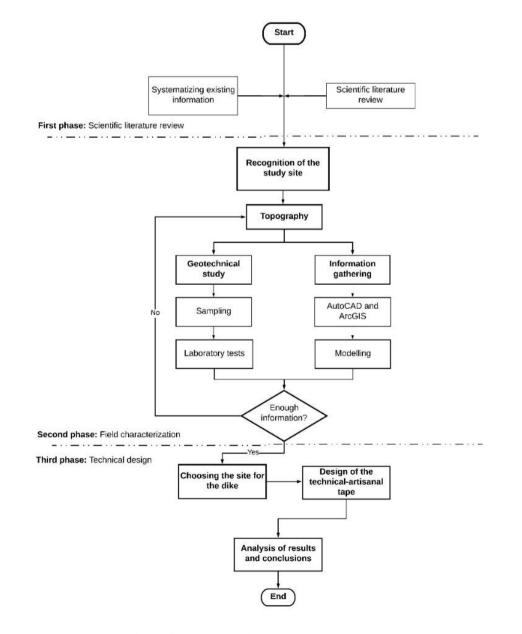


Figure 2. Flowchart of the research methodology

The topographic study was carried out on the entire reservoir area of the chosen site to find the ideal area that maximises storage efficiency and minimises construction cost. Thanks to the studies of [32, 36] several factors have been analysed in the sub-basin of the Manglaralto river, such as geomorphology (slopes and relief), geology-hydrogeology, and considering the river narrowing and course. Placing the dike in a narrowing of the river guarantees a maximum of water storage (because the river widens upstream) and a minimum construction cost (because the dike is less wide in a narrowing, which translates into less material use).

The topography was carried out with differential GPS equipment. In the office, the field data obtained in specialised software, such as AutoCAD Civil 3D and ArcGIS, were entered. The possible volume of water that it could store was also estimated.

3.2 Geotechnical characterisation

For the dike (tape) design, it was necessary to know the soil characteristics on which it will be built. Therefore, soil samples were taken at strategic points, according to terrain's morphology, such as the sides of the slopes and the reservoir area. With this, a better representation of the soil on which the dike (tape) will be built was obtained.

For the characterisation of the soil samples, tests of granulometry, Atterberg limits [38], Triaxial [39], Los Angeles abrasion [40] and resistance to compression were performed. All tests were carried out in a laboratory specialised in soil mechanics. For the granulometry, Atterberg limits and triaxial tests, soil samples were taken from each face of the slope, for which the ground was first cleared. Then, with the help of a Shelby tube, soil samples were cored. The material's soil class, its plasticity, cohesion, and internal friction angle were known with these tests. With these results, the dike's embedment in field was designed. Los Angeles abrasion trial's purpose was to determine if the material located at the bottom of the river or the banks could be used or not as a construction material. If the value is less than 40% in the Los

Angeles Abrasion test, a concrete cylinder was made to test its resistance to compression later.

3.3 Design of the technical-artisanal dike

In this third phase of the methodology, the design and model of the dike was elaborated, always with the objective and purpose of maximising efficiency, minimising costs and prolonging its useful life. The results obtained from the geotechnical tests carried out on the samples obtained in the area chosen for the dike were analysed. The geometric characteristics and the most suitable materials to have and use in the dike were defined with these results. A detail of the dike design and a diagram of the proposed dike in the study sector are presented. Additionally, a referential budget calculation of the cost of this civil work is presented.

4. RESULTS

4.1 Choosing the site for the dike (tape)

The chosen site is located in the Manglaralto River subbasin, with coordinates UTM (17S) 529564 E; 9796588 S (see Figure 3). This site was chosen thanks to the data provided by [32, 36], which indicate it as a geologically strategic sector for creating a dike. The factors considered for determining the location of the dike were: geomorphology, hydrogeology, tectonics of the study site, proximity to water wells, and distance to the coastline. With this, the favourable conditions of the subsoil that this place presents for the storage, recharge and infiltration of water to the existing aquifer in the sector is guaranteed. Thanks to the infiltration and recharge of water to the aquifer, it is possible to reduce saline intrusion in these wells near the coastline.

After carrying out the data collection for the terrain topography, we proceeded with their analysis. The digital model can be seen in Figure 4. Two critical parameters for the dike design were determined: the dike geometry and the water volume retained in the reservoir.

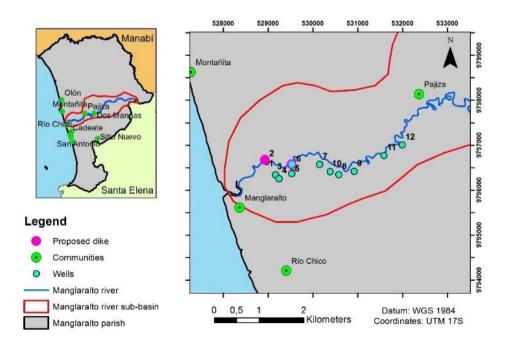


Figure 3. Dike (tape) location map

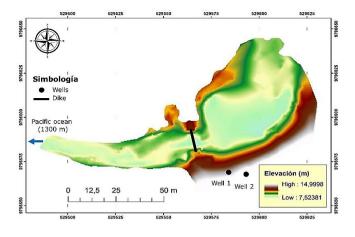


Figure 4. Modelling the area of interest. (Source: [41])

4.1.1 Dike geometry and volume of water retained

It must be as narrow as possible for economic reasons, and it cannot exceed the reservoir's neck level so that the water does not escape from its sides. Thus, the estimated height was 3 meters, the same as that calculated by subtracting the highest elevation from the lowest elevation on both sides of the neck. The length was calculated so that the minimum distance reached the highest elevation that the riverbed would reach on both sides. This length is 15.45 m (not counting embedment of the dike).

The water volume calculation was carried out considering the reservoir as a closed reservoir using contour lines and the river level's average height, resulting in 4641.88 m³.

4.2 Geotechnical characterisation

Samples were taken at 5 points: the two sides of the foundation (on the slopes) and the rest in the reservoir area (in the river, at a depth of 2 m). Table 1 shows the coordinates where the samples were taken. The samples were taken on the slopes (on the banks of the river) help us to design the embedment of the dike and its stability correctly. While the samples were taken along the riverbed help us know the water storage capacity and its infiltration-recharge to the aquifer.

Table 1. Location and tests performed on each sample

Sample	Coordinates		TT	D
	X	у	Test	Description
P1	529.558	9'796.592	Granulometry, Atterberg limits, Triaxial	Left slope
P2	529.564	9'796.580	Granulometry, Atterberg limits, Triaxial	Right slope
Р3	529.529	9'796.577	Granulometry, Atterberg limits, Triaxial, Los Angeles abrasion	Excavation in river
P4	529.545	9'796.581	Los Angeles abrasion	Excavation in river
P5	529.569	9'796.589	Los Angeles abrasion	Excavation in river

The granulometry of the left slope determined that the material is composed of well-graded sand with some silt, and, for the right slope, it is a mixture of poorly graded gravel with a considerable percentage of coarse sand. Similarly, the excavation sample was carried out, consisting of coarse gravel with a low percentage of fines (ratio 80% coarse and 20% fine). The results obtained for Atterberg limit tests to determine the soils' plasticity, detailed in Table 2.

Tests	Left slope	Right slope
Liquid limit	38.70	62.40
Plastic limit	31.20	43.73
Plasticity index	7.40	18.31

From the triaxial, cohesion and internal friction angle were obtained, both for the left slope and the right slope, as shown in Table 3. From the Los Angeles abrasion test, it was obtained that the wear percentage is 32.82%. As an additional test, the concrete specimens were elaborated, from which it was obtained that the concrete made with coarse aggregate obtained a resistance of 277.8 Kg/cm².

Table 3. Results of the triaxial tests

Tests	Left slope	Right slope
Cohesion	54.92 kPa	348.07 kPa
Internal friction angle	37.23°	20.30°

4.3 Design of the technical-artisanal dike

The dike (tape) design was made based on the results obtained, considering the characteristics established below:

(1) The dike must be made up of a very well compacted gravel-sandy embankment with an average diameter of 1", which may consist of the material found in the river.

(2) The embankment must have a trapezoidal section, with a base greater than 7.00 m, a baseless than 2.00 m and a height of 2.50 m.

(3) The embankment must be covered by a reinforced concrete covering f'c= 360 Kg/cm^2 , a thickness of e=0.50 m. The coarse aggregate used to make this concrete can be obtained from the material found in the riverbed.

(4) The concrete finish must be perfectly smooth.

(5) A ridge 0.50 m thick and 1.00 m high will be formed by burying the concrete in the natural terrain.

(6) The concrete must contain an electro-welded mesh of 20x20 cm, $\dot{0}$ 8mm, leaving 5 cm of covering on each side.

(7) To guarantee the dike's stability, the embedments must be inserted 7 m on each edge of the river, thus avoiding any possibility of erosion on the dike's sides.

(8) After the dike's embedments have been built, the channel's edges must be rebuilt, using a mixture of clay soil-Type I Portland cement with a 3:1 ratio.

(9) The soil must be compacted in layers of 0.25 m at 95% of the standard Proctor in the upper part.

The dike design (Figure 5) was carried out based on the results obtained, considering the characteristics established below. Considering the characteristics and construction guidelines given above, the civil works' total cost is estimated at USD 94,662.53. 96% represents the dike structure's construction (Concrete f'c = 360 Kg/cm^2 and electro-welded mesh 20x20, \acute{O} 8mm), and the rest represents the cost of earthworks.

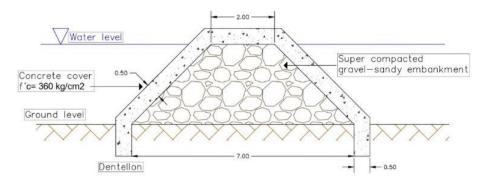


Figure 5. A cross-sectional view of the dike. (Source: [41])

5. ANALYSIS OF RESULTS

The study site's choice is considered the most optimal because it is located near two wells owned and managed by JAAPMAN (well 1 and 2 in Figure 3). This would be beneficial, thanks to the accumulation of water in that sector of the river, which would generate the aquifer's recharge used to supply Manglaralto [2, 26]. Thanks to the study sector's digitised model (Figures 4 and 5), the tape's excellent location was verified; since there would be a large reservoir and a narrow area, which minimises the work's cost maximises storage. For the construction of the dike, an embedment of not less than 5 m was established because, from the granulometric tests, it was possible to determine that the slopes on which the structure is based consist of materials not suitable for embedment since the slope the left consists of fine sand and the right consists of fine gravel.

Thanks to the Los Angeles abrasion and compression resistance tests, it was obtained that the material can be used as an aggregate for the concrete that will line the dike since the maximum allowable wear is 40% according to the ASTM C131 standard. and ASTM A535 [42]. Also, it reaches the permitted resistance range ($\pm 10 \text{ Kg/cm}^2$), directly benefiting the referential budget for civil works. For the construction of the dike, although in the laboratory tests, the concrete has been dosed to obtain a resistance f'c = 280 Kg/cm^2 , which, to design the structure is efficient, it is recommended that the resistance of the concrete let $f'c = 360 \text{ Kg/cm}^2$. This in order to increase its resistance against water erosion and to be able to resist the blow of blocks that may impact the structure, in addition to the fact that the concrete finish must be perfectly smooth, in such a way that it facilitates the flow of water together with the materials that this drag.

Carrión et al. [26], you can see the benefits that the construction of this reinforced concrete dam design has brought. A case similar to that of Manglaralto, in which dikes were built to artificially recharge the aquifers to supply the populations of sectors where access to water is problematic, is that of the hydrological correction dam of the Quípar River located in Spain. Located in a semi-arid region where surface water resources are scarce; in this case, there are other dams in Spain, as in the country dams have been built to retain water to recharge the associated aquifers since the end of the 20th century [43]. The artificial recharge of aquifers is a subject that has been widely studied and treated in several countries such as Libya, Algeria, Egypt, Sudan, Portugal, Spain, Greece, Germany, Finland, among others. This requires artificial techniques such as the construction of dams to be able to solve its hydrological deficit and in this way obtain indirect benefits such as reducing poverty, reducing health risks, increasing people's standard of living and, among other benefits, increasing the sustainability of the place.

The aquifers receive direct benefits thanks to the construction of dikes, and in the case of Manglaralto, this dam not only helps to retain surface water and increase the amount of groundwater but also prevents the salinity that affects the material of the subsoil [27] contaminates the well due to its location near the sea. In addition to this, the Manglaralto aquifer is considered by Herrera et al. [44, 45] a site of geological interest, so by allowing its recharge, it is giving way to its conservation and preservation.

The advantages obtained due to the dike's construction are economic, social, environmental and cultural. In the economic aspect, the use of river aggregates is highlighted to reduce the cost of construction. In addition, thanks to the formed reservoir, the community practices fishing and tourism. In the social aspect, it can be noted that this reservoir area generates a recreation area for the community and tourists. In the environmental aspect, this reservoir allows the growth of the area's biodiversity and its surroundings. Furthermore, in the cultural aspect, the ancestral knowledge of the community is rescued. Previously, so that the river water does not escape into the sea, tapes (accumulation of soil in a part of the river) were made. This impact is being replicated in the communities near Manglaralto, such as Olón [4], Cadeate [6, 46] and Libertador Bolívar [47]. In these communities, engineering studies are being carried out in the basins to see the feasibility of constructing dikes.

6. CONCLUSIONS

The technical-artisanal dike (tape) was designed to allow optimal surface storage and artificial recharge of the aquifer to provide safe water to the Manglaralto community. This dike's dimensions are a total length of 30 m (16 m free length and 7 m embedment) and a height of 3 m (from the ground level to the covering). It has a reinforced concrete covering 0.5 m thick with a resistance of 360 Kg/cm². A dentil of 0.5 m thick and 1 m high, an electro-welded mesh of 20x20 cm, \acute{O} 8mm, a 3:1 soil-cement mixture for the reconstruction of the river edges and compaction of 95% of the standard Proctor in the upper part of the embedments.

The designed structure will allow the overflow of streams in the ample avenues while achieving impoundment and retention of the subsurface flow in the alluvial deposit.

The referential budget for civil works is USD 94,662.53, which 96% represents the structure of the dike, and the rest is

earth movement. An opportunity for the dike's construction is that the cost per earth movement for the work is considerably reduced because the river's material can be used as an aggregate for the concrete.

After all the pertinent considerations exposed in this research, such as the social, economic and environmental impact, it can be said that the construction of the dam, as part of the solution to the problem of water scarcity of the Manglaralto Parish, is feasible.

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NOMENCLATURE

"	inches
°C	Degree Celsius
Ó	Steel diameters
ASTM	American Society for Testing and Materials
cm	Centimeter
cm ²	Square centimeter
e.g.	Exempli gratia
f'c	Concrete Compressive strength
GPS	Global Positioning System
kg	kilogram
km ²	Square kilometers
m	Meters
m ³	Cubic meters
mm	Millimeters
USD	United States Dollar
UTM	Universal Transverse Mercator