

Empirical Relationships on Mechanical Properties of Class-F Fly Ash and GGBS Based Geopolymer Concrete

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ABSTRACT

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Ordinary Portland Cement (OPC) is one of the important materials used in the development of the construction industry. Production of OPC consumes lot of raw materials, emits extreme quantity of carbon dioxide (CO₂) into atmosphere and highly energy-intensive. Thus, to reduce the production of cement so many attempts are done and an alternative binder was found. The alternative to cement concrete is geopolymer concrete (GC) and it is produced by mixing industrial by-products (rich in aluminosilicate) with an alkaline solution. This paper explores the mechanical properties and a representative non-linear equation was proposed for the relationship between splitting tensile Vs compressive strengths and flexural Vs compressive strengths of fly ash-GGBS synthesized GC cured at ambient temperature for 28 days. This work also presents empirical formulae for predicting elastic modulus of fly ash-GGBS synthesized GC. Equations were proposed to determine the splitting tensile strength, flexural strength and elastic modulus based on the compressive strength of GC. The obtained results are clearly indicating that the predicted (from proposed equations) splitting tensile and flexural strength values are very close to the experimental values. It is established that the formulae found in various codes and literature generally predict the higher values of the elastic modulus than those obtained using the proposed equation.

1. INTRODUCTION

Geopolymer Concrete (GC) is an emerging alternative to conventional cement-based concrete with an eco-friendly environment. It possible to improve sustainable utilization of industrial by-products in construction [1-2]. Several investigations have been done on the improvement of GC synthesized from dissimilar aluminosilicate source materials [2-7]. GC is produced by the reaction of different aluminosilicate materials such as fly ash and Ground Granulated Blast Furnace Slag (GGBS) with an alkaline solution [7-8]. Use of industrial by-products in the making of GC can contribute to the reduction of CO₂ emissions caused by cement production [9-10]. The micro-structural growth and mechanical properties of GC in geo-polymerization are mainly depending on the chemical composition of by-products and the type of alkaline solution used to produce geopolymers [11-12]. OPC or PPC cement mainly form C-S-H gel, while geopolymer resin mainly consists of an aluminosilicate gel. This difference shows the outstanding advantages of geopolymer binder over the OPC binder [13]. GC has been reported to obtain superior engineering properties and durability characteristics, also as good as ordinary concrete in several cases of structural applications [14-21].

The state of the reports on GC undoubtedly indicated that purely fly ash synthesized GC requires additional supply of heat (60-90 °C temperature of curing) for a minimum of 24 hours to attain required strength [22-24]. With the inclusion of GGBS or any other industrial by-product, it is understood that the supply of heat can be eradicated and can achieve

necessitated strength properties [22]. The flexural and splitting tensile strengths of GC samples are favorably related to that of conventional concrete specimens and also noted that strength parameters of GC samples are depending on the mix proportions and curing conditions [25]. Engineering properties of GC activated with different activators and a silicate ion present in the activator solutions enhanced strength and young's modulus values [26]. The elastic modulus of concrete is an important factor to evaluate construction applications and service life [5]. Elastic modulus of OPC concrete ranges between 30300-34500 MPa, whereas in GC it was found that 10700-18400 MPa [26]. Young's modulus of low-calcium based geopolymer specimen's ranges between 23000-30800 MPa [27]. Alkali activated pulverized fuel ash binder has lower young's modulus values compared to OPC as well as geopolymer binders [28]. Initial 14 days the moduli of elasticity for alkali-activated pozzolan specimens are lower compared to OPC specimens and in a longer time, the values are increased up to 10-20 % [29]. Hence a huge dissimilarity in the elastic modulus of GC was identified in the earlier literature.

2. MATERIALS

2.1 Binders

Fly ash is the by-product produced from coal burning in the generation of electricity like thermal power plants [30]; annually more than 220 million tonnes of fly ash is producing in India [31]. Fly ash used in this research was taken from

NTPC-Vijayawada, India and GGBS were taken from JSW Cements Ltd, Vijayawada, India. The oxide composition of Fly ash and GGBS was determined by XRF analysis is shown in Table 1. Specific gravity and specific surface area of different materials used in this paper are shown in Table 2.

2.2 Alkaline solution

The mixture of sodium silicate and sodium hydroxide in a mass ratio of 2.0 was used as an alkaline solution. NaOH in flakes form obtained from Vamshi Krishna chemical solutions, Vijayawada and Na₂SiO₃ in solution form taken from Kiran Global Solutions, Hyderabad are used. The concentration of NaOH solution was maintained as constant i.e., 8M. Alkaline activator solution will be prepared at least 24 hours before casting the specimens to get desired results [22]. In order to achieve higher strength geopolymer concrete, the most advantageous range of sodium silicate to sodium hydroxide was 0.67 to 1.00 [32]. Economic point of view sodium silicate to sodium hydroxide ratio and binder to alkaline activator solution percentage maintained as 2.0 and 0.40, respectively for all the mixes in this research work. Alkaline solution (Sodium hydroxide solution mixed with sodium silicate solution) should be prepared one day before making of GC specimens to get desired results [33].

2.3 Aggregates

Aggregates are one of the principal components of concrete since it occupies almost three-quarters of the volume of concrete. The quality of aggregates used in concrete invariably affects the quality of the final concrete. In this paper, Krishna river sand in the surface dry condition is used as fine aggregate and coarse aggregates are sourced from local dealers in Guntur, A.P., India. The sieve analysis tests were carried out according to IS 2386–1963 (Part I) [34] on aggregates to determine the conformation to IS 383–2016 [35]. The coarse aggregates of less than 20mm size were used in all mixes of GC. Sieve analysis is conducted on the mixed coarse aggregate sample. The result obtained on sieve analysis of coarse and fine aggregates is presented in Table 3 and Table 4, respectively.

2.4 Mix proportions of GC

The mix proportion for M20 grade of GC is carried out according to the previous mix design procedures and Indian standard code [36-38]. Table 5 shows the mix proportioning of different materials used in this research work.

Table 1. Major oxide composition of fly ash and GGBS determined by XRF analysis

Oxide	Al ₂ O ₃	CaO	Fe ₂ O ₃	SiO ₂	P ₂ O ₅	SO ₃	MgO	Na ₂ O	K ₂ O	TiO ₂	MnO	LOI
Fly ash	25.08	2.87	4.56	58.23	0.2	1.16	1.21	0.41	0.87	0.83	2.94	1.59
GGBS	12.14	44.71	1.10	32.25	-	0.84	4.23	0.87	-	-	1.96	1.98

Table 2. Properties of different materials (specific gravity and specific surface area)

Material	Fly ash	GGBS	Sodium hydroxide	Sodium silicate	Coarse aggregates	Fine aggregates	Water
Specific Gravity	2.31	2.8	2.13	1.53	2.8	2.63	1
Specific surface area (m ² /kg)	367	508	-	-	-	-	-

Table 3. Sieve analysis of coarse aggregate

IS sieve size (mm)	Weight retained (gm)	Percentage retained	Cumulative % retained	Cumulative % passing	Limit as per IS 383–2016	Remarks
40	0	0	0	100	100	Aggregates conform to 20mm nominal maximum size CA
20	1107	11.07	11.07	88.93	85-100	
10	7060	70.60	81.70	18.3	0-20	
4.75	1816	18.16	99.86	0.14	0-5	
Pan	14	0.14	100	0	0	

Table 4. Sieve analysis of fine aggregate

IS sieve size (mm)	Weight retained (gm)	Percentage retained	Cumulative % retained	Cumulative % passing	Limit as per IS 383–2016	Remarks
10	0	0	0	100	100	Aggregates conform to Zone II of IS: 383 – 2016
4.75	14	1.4	1.4	98.6	90-100	
2.36	181	18.1	19.5	80.5	75-100	
1.18	184	18.4	37.9	62.1	55-90	
0.6	251	25.1	63	37	35-59	
0.3	193	19.3	82.3	17.7	8-30	
0.15	148	14.8	97.1	2.9	0-10	

Table 5. Mix proportions of different materials (kg/m³)

Mix Id	Fly ash	GGBS	Fine aggregate	Coarse aggregate	Sodium hydroxide (8M)	Sodium silicate	Alkaline solution/Binder (s/b)
F ₁₀₀ G ₀	407	0	610.5	1221	54.26	108.53	0.4
F ₉₀ G ₁₀	366.3	40.7	610.5	1221	54.26	108.53	0.4
F ₈₀ G ₂₀	325.6	81.4	610.5	1221	54.26	108.53	0.4
F ₇₀ G ₃₀	284.9	122.1	610.5	1221	54.26	108.53	0.4
F ₆₀ G ₄₀	244.2	162.8	610.5	1221	54.26	108.53	0.4
F ₅₀ G ₅₀	203.5	203.5	610.5	1221	54.26	108.53	0.4
F ₄₀ G ₆₀	162.8	244.2	610.5	1221	54.26	108.53	0.4
F ₃₀ G ₇₀	122.1	284.9	610.5	1221	54.26	108.53	0.4

2.5 Mixing and casting

Fly ash, other industrial by-products (like GGBS, Silica fume, Red mud, Zeolite), and alkaline solution were mixed mutually into a homogenous binder. The binder was then introduced into the dry sand and mixed for 30-40 seconds. Afterward, aggregates of size less than 20 mm were added to the mixture and uniformly mixed for 2-2.5 minutes. After mixing of all these materials, GC was transferred into moulds (cube, cylinder and beam) and Due to less setting time of GC, a vibration machine was used to eliminate air attentive in the specimens.

2.6 Workability of GC

The workability of fresh GC mixtures was calculated instantaneously after mixing by a slump cone test according to IS 1199-1959 [39]. An Abrams slump cone of dimensions was 300 mm in height, and 100 mm and 200 mm in diameters at the top and bottom, respectively. It was observed that workability of GC slightly increased by adding 0.75 % of water (i.e. 60 ml) for all mixes. All mixes of GC were a medium range of degree of workability was observed and workability values are less than 75 mm is shown in Table 6.

Table 6. Workability of Fly ash-GGBS based GC

Mix Id	Slump value	Degree of workability
F ₁₀₀ G ₀	82	Medium
F ₉₀ G ₁₀	75	
F ₈₀ G ₂₀	68	
F ₇₀ G ₃₀	55	
F ₆₀ G ₄₀	54	
F ₅₀ G ₅₀	55	
F ₄₀ G ₆₀	51	
F ₃₀ G ₇₀	51	

2.7 Curing and testing

Compression testing (2000 kN capacity) and flexural testing machines (100 kN) are used to find the mechanical properties of GC. To determine the compressive strength of GC cubes Indian standard code was used IS 516 (1959) [40] Splitting tensile strength of cylindrical specimens is carried according to the prescribed specifications of ASTM C496/C496M-17 [41]. Flexural strength of prism/beam specimens is tested with Third-Point Loading as per American standard test procedure ASTM C78/C78M-18 [42]. There is no necessity of exposing GC specimens to elevated temperatures to gain maximum strength if the smallest amount (9 % minimum) of fly ash is replaced by GGBS. In this paper,

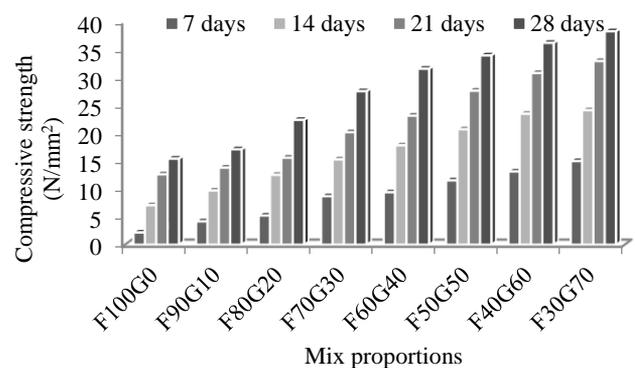
ambient curing condition was followed to cure the specimens at a temperature of 32±3 °C for 7, 14, 21 and 28 days.

3. MECHANICAL PROPERTIES

The mechanical properties of GC are increased by an increase in the quantity of fly ash fineness [43]. The fineness (in terms of a specific surface) of fly ash taken in this work was 367 m²/kg, to improve the strength properties of GC. Fly ash and GGBS based GC showed greater mechanical properties while increasing the GGBS (up to 70 %) content strength was also increased [14-15,44-45]. Setting time of pure fly ash-based GC at room temperature may take more than 40 hours to set [46]. Thus, it was observed that the addition of GGBS in fly ash-based GC will help to improve the setting time and strength.

3.1 Compressive strength

Standard cubical moulds (150×150×150 mm) are used to determine the compressive strength [40] of GC for 7, 14, 21 and 28 days of ambient curing. The obtained results at dissimilar proportions of GC are shown in Figure 1. The target strength obtained in mix design is 26.6 N/mm², but for GC at a mix proportion F₃₀G₇₀ given highest compressive strength i.e. 38.34 MPa at 28 days of ambient curing.

**Figure 1.** Compressive Strength of GC at different mix proportions and curing ages

3.2 Splitting tensile strength

Cylindrical specimens (300×150 mm) are used to know the strength of GC with different mix proportions and curing ages. Generally, the tensile strength of conventional concrete varies between 10-15 % of compressive strength i.e. tensile strength of M20 grade concrete is 2-3 MPa. It was observed

that GC specimens are gaining greater tensile strength up to 15-20 % of compressive strength i.e. 2-6 MPa for 28 days of ambient curing. 30 % Fly ash and 70 % GGBS based GC specimens showed 6.68 MPa splitting tensile strength at 28 days of curing (Figure 2).

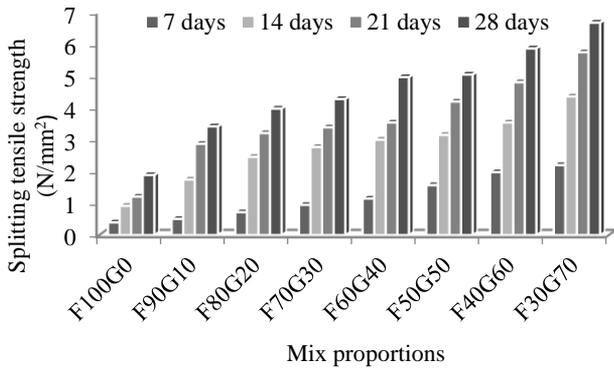


Figure 2. Splitting tensile strength of GC at different mix proportions and curing ages

3.3 Flexural strength

To determine the flexural strength of GC beam moulds of size 100×100×500 mm was used and rate of load applied at a rate of loading of 400 kg/min. Conventional concrete flexural strength varies between 15-25 % of compressive strength i.e. for M20 grade is in between 3-5 MPa (Figure 3).

$$f_b = \frac{3pl}{2bd^2}$$

where, f_b =Flexural Strength, p = Ultimate load (KN), l = Supported length (mm), b = Width of the beam specimen (mm), d = Depth of the beam specimen (mm).

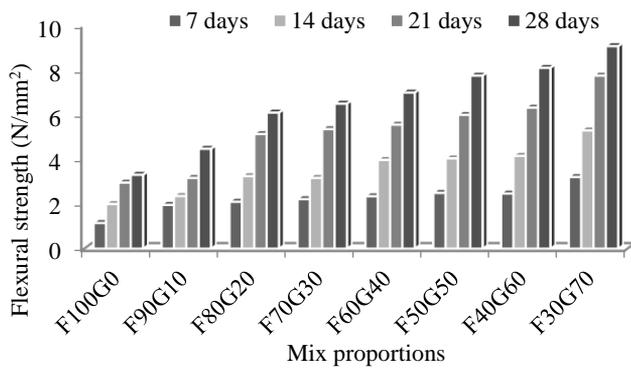


Figure 3. Flexural strength of GC at different mix proportions and curing ages

3.4 Comparison between predicted and experimental values of GC

According to several codes and researchers splitting tensile and flexural strengths of concrete is strongly associated with that of compressive strength [47-57]. The correlation between splitting tensile strength and compressive strength of concrete can be represented by non-linear equations because the tensile or flexural strengths of concrete increases with an increase in compressive strength and the ratio of tensile

strength to compressive strength decreases as the compressive strength increase [58]. This implies that regression analysis using the least square fit method was adopted for the non-linear relationship between split tensile, flexural strengths of GC from compressive strength. Table 7 and 8 shows the predicted values of splitting tensile strength and flexural strength of GC from codes, literature and proposed equation.

3.5 Predicted splitting tensile and flexural strengths from experimental compressive strength

Design of concrete structures recommended certain standard equations to predict the splitting tensile and flexural strength from compressive strength. The recommended equations in the American Concrete Institute (ACI) and some other research papers were used to predict splitting tensile and flexural strengths of GC specimens. The ACI 363R-92 [48] and ACI 318-99 [49] recommends respective Eq. (1) and Eq. (2) as the appropriate relationship between the splitting tensile strength and compressive strength. Similarly, Eq. (3) and Eq. (4) recommends an approximate relationship between flexural strength and compressive strength. Not only ACI code some previous research papers also proposed few equations in associated with splitting tensile, flexural strengths from compressive strengths are shown in Figure 4 and 5. The proposed Eq. 5 and Eq. 6 shows the relationship between splitting tensile, flexural strengths of GC from compressive strength.

$$f_t = 0.59\sqrt{f_c} \quad (1)$$

$$f_t = 0.56\sqrt{f_c} \quad (2)$$

$$f_{ft} = 0.94\sqrt{f_c} \quad (3)$$

$$f_{ft} = 0.62\sqrt{f_c} \quad (4)$$

$$f_t = 0.51f_c^{0.66} \quad (5)$$

$$f_{ft} = 0.57f_c^{0.74} \quad (6)$$

where, f_t = Splitting tensile strength (Predicted), f_{ft} = Flexural strength (Predicted), f_c =Compressive strength (Experimental)

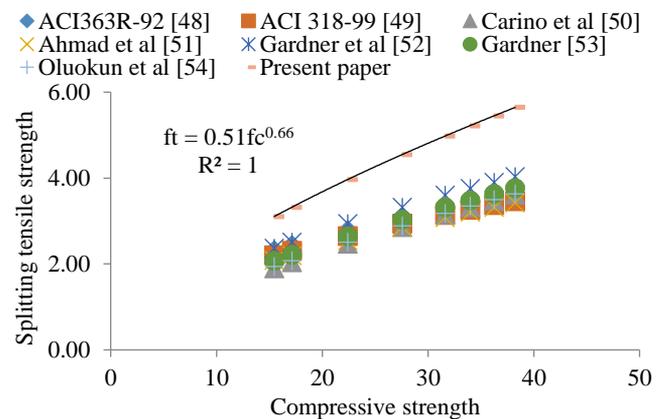


Figure 4. Comparison of predicted splitting tensile strength values

Table 7. Predicted splitting tensile strength for several relationships

Mix Id	Experimental data		Predicted data							
			ACI363R-92 [48]	ACI 318-99 [49]	Carino et al [50]	Ahmad et al [51]	Gardner et al [52]	Gardner [53]	Oluokun et al [54]	Present paper
	Compressive strength (N/mm ²)	Splitting tensile strength (N/mm ²)	$f_t=0.59f_c^{0.5}$	$f_t=0.56f_c^{0.5}$	$f_t=0.272f_c^{0.71}$	$f_t=0.462f_c^{0.55}$	$f_t=0.47f_c^{0.59}$	$f_t=0.34f_c^{0.66}$	$f_t=0.294f_c^{0.69}$	$f_t=0.51f_c^{0.66}$
F ₁₀₀ G ₀	15.46	1.89	2.32	2.20	1.90	2.08	2.36	2.07	1.94	3.11
F ₉₀ G ₁₀	17.14	3.42	2.44	2.31	2.04	2.2	2.51	2.21	2.08	3.32
F ₈₀ G ₂₀	22.43	3.98	2.79	2.65	2.47	2.55	2.94	2.64	2.51	3.97
F ₇₀ G ₃₀	27.58	4.28	3.09	2.94	2.86	2.86	3.32	3.03	2.89	4.55
F ₆₀ G ₄₀	31.61	4.97	3.31	3.14	3.15	3.08	3.6	3.32	3.18	4.98
F ₅₀ G ₅₀	34	5.05	3.44	3.26	3.32	3.21	3.76	3.48	3.35	5.22
F ₄₀ G ₆₀	36.25	5.87	3.55	3.37	3.48	3.32	3.9	3.63	3.5	5.45
F ₃₀ G ₇₀	38.24	6.68	3.65	3.46	3.62	3.43	4.03	3.77	3.63	5.65

Table 8. Predicted flexural strength for several relationships

Mix Id	Experimental data		Predicted data					
			ACI363R-92 [48]	ACI 318-99 [49]	Mhaiskar and Naik [55]	M. Ahmed [56]	M. Irwan Juki [57]	Present paper
	Compressive strength (N/mm ²)	Flexural strength (N/mm ²)	$f_{fs}=0.94f_c^{0.5}$	$f_{fs}=0.62f_c^{0.5}$	$f_{fs}=0.864f_c^{0.5}$	$f_{fs}=1.055f_c^{0.5}$	$f_{fs}=0.466f_c^{0.703}$	$f_{fs}=0.57f_c^{0.74}$
F ₁₀₀ G ₀	15.46	3.31	3.70	2.44	3.40	4.15	3.19	4.32
F ₉₀ G ₁₀	17.14	4.48	3.89	2.57	3.58	4.37	3.43	4.67
F ₈₀ G ₂₀	22.43	6.1	4.45	2.94	4.09	5.00	4.15	5.69
F ₇₀ G ₃₀	27.58	6.5	4.94	3.26	4.54	5.54	4.80	6.64
F ₆₀ G ₄₀	31.61	7	5.28	3.49	4.86	5.93	5.28	7.34
F ₅₀ G ₅₀	34	7.76	5.48	3.62	5.04	6.15	5.56	7.75
F ₄₀ G ₆₀	36.25	8.11	5.66	3.73	5.20	6.35	5.82	8.12
F ₃₀ G ₇₀	38.24	9.08	5.81	3.83	5.34	6.52	6.04	8.45

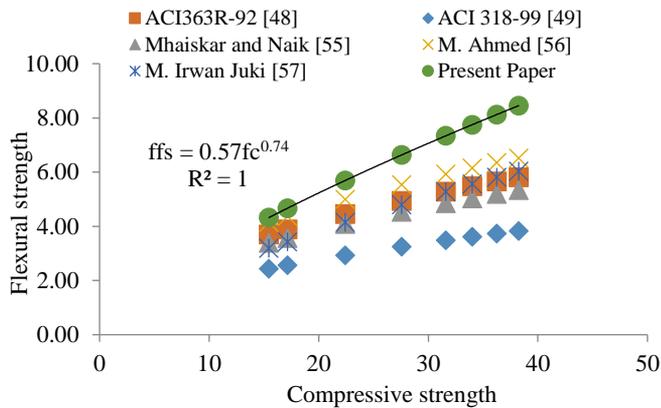


Figure 5. Comparison of predicted flexural strength values

4. MODULUS OF ELASTICITY

Young’s modulus measures the resistance of any material against elastic deformation when a load is applied. The Young’s modulus of GC was tested according to ASTM C469/C469M-10 [47]. The experiment was done with cylindrical specimens of 150 mm in diameter and 300 mm in height and three specimens are tested at each age. The mean value of the elastic modulus for 28 days is determined for GC. Generally, young’s modulus values vary with respect to compressive strength. With the increases of compressive strength of concrete young’s modulus values of concrete will also be increasing. Young’s modulus of GC values is relatively low compared to conventional concrete of parallel compressive strength [59]. According to IS 456-2000 [60] the

modulus of elasticity of conventional concrete (M20) for 28 days is 22360.68 MPa. Whereas modulus of elasticity of different mixes (F₁₀₀G₀, F₉₀G₁₀, F₈₀G₂₀, F₇₀G₃₀, F₆₀G₄₀, F₅₀G₅₀, F₄₀G₆₀ and F₃₀G₇₀) based GC ranges between 13362-20196.34 MPa. The experimental data shows that young’s modulus of GC was 20-40 % less compared to conventional concrete.

Oven cured fly ash based GC specimens are reported to have less young’s modulus values as compared to conventional concrete [25-26]. Oven cured fly ash based GC specimens of average compressive strength about 55 MPa showed young’s modulus values between 14.9-28.8 % lower than conventional concrete [61].

Comparing the modulus of elasticity of GC specimens, much significant difference was observed due to variation in the GGBS content. By the increase of GGBS content in the GC mix modulus of elasticity was also increased. Pure fly ash-based GC specimens were showed lower compressive strength as well as young’s modulus at ambient curing.

The experimental data are compared with predicted young’s modulus values by the equations proposed in different codes and by earlier research papers are as given below.

(i) Based to the ACI 318-14 (ACI Building Code) [62] the young’s modulus of conventional concrete (density ranges from 1442 to 2483 kg/m³) can be determined by using Eq. (7).

$$E_c = 0.043 \times \rho^{1.5} \times \sqrt{f_c} \quad (7)$$

where, E_c = Young’s Modulus (MPa); ρ = Concrete Density (kg/m³); f_c = Concrete Compressive strength (MPa), after 28 days of curing.

(ii) The young's modulus of conventional concrete can be determined by the CEB-FIP [63] code by using Eq. (8).

$$E_c = 0.85 \times 2.15 \times 10^4 \times \sqrt[3]{\frac{f_c}{10}} \quad (8)$$

(iii) According to the Indian Standard Building Code IS 456-2000 [60], young's modulus of normal weight conventional concrete can be estimated by Eq. (9).

$$E_c = 5000 \times \sqrt{f_c} \quad (9)$$

(iv) Based on experimental results oven cured fly ash based GC, Hardjito et al. [27] proposed Eq. (10) to determine the young's modulus values.

$$E_c = 2707 \times \sqrt{f_c} + 5300 \quad (10)$$

(v) To predict young's modulus of GC, Diaz-Loya et al. [64] proposed Eq. (11).

Therefore, the strength at a particular age has been considered as the value of compressive strength, while determining the young's modulus of GC using Eq. (11).

$$E_c = 0.037 \times \rho^{1.5} \times \sqrt{f_c} \quad (11)$$

(vi) Lee and Lee [65] proposed Eq. (12) to predict the young's modulus of geopolymer concrete.

$$E_c = 5300 \times \sqrt[3]{f_c} \quad (12)$$

(vii) Nath and Sarker [59] proposed Eq. (13) to predict the young's modulus of ambient-cured low-calcium fly ash blended GC.

$$E_c = 3510 \times \sqrt{f_c} \quad (13)$$

The obtained result shows that the predicted model to determine young's modulus of fly ash-GGBS based GC from compressive strength; the following Eq. (14) was proposed. The proposed equation to determine the modulus of elasticity of GC is shown in Figure 6.

$$E_c = 3282 \times \sqrt{f_c} \text{ MPa} \quad (14)$$

The results of elastic modulus of 100 % fly ash and fly ash-GGBS based geopolymer concrete from codes; previous literature and the present study are shown in Table 9.

It should be noted that, compared to fly ash-based GC fly ash-GGBS synthesized GC specimens are shown greater elastic modulus values.

Table 9. Elastic modulus of GC from codes, literature and present study

Mix Id	Compressive Strength (MPa)	Mean Density of GC (kg/m ³)	Young's Modulus (E) MPa								
			Experimental (28 days)	ACI 318-14 [62]	CEB-FIP Model [63]	IS 456-2000 [60]	Hardjito et al [27]	Diaz-Loya et al [64]	Lee and Lee [65]	Nath and Sarker [59]	This paper
F ₁₀₀ G ₀	15.46	2310	13362.00	18771.16	21128.28	19659.60	15943.71	16151.93	13191.15	13801.04	12904.56
F ₉₀ G ₁₀	17.14	2356	14297.30	20358.08	21866.69	20700.24	16507.11	17517.42	13652.16	14531.57	13587.64
F ₈₀ G ₂₀	22.43	2377	15768.21	23600.81	23915.72	23680.16	18120.44	20307.67	14931.45	16623.47	15543.66
F ₇₀ G ₃₀	27.58	2375	17001.05	26137.32	25619.77	26258.33	19516.26	22490.25	15995.35	18433.35	17235.97
F ₆₀ G ₄₀	31.61	2378	18164.60	28034.87	26810.13	28111.39	20519.50	24123.02	16738.53	19734.19	18452.31
F ₅₀ G ₅₀	34.00	2389	18968.75	29277.38	27468.81	29154.76	21084.39	25192.16	17149.77	20466.64	19137.18
F ₄₀ G ₆₀	36.25	2385	19520.62	30154.70	28061.25	30103.99	21598.30	25947.07	17519.65	21133.00	19760.26
F ₃₀ G ₇₀	38.24	2392	20196.34	31107.79	28565.11	30919.25	22039.68	26767.17	17834.23	21705.31	20295.40

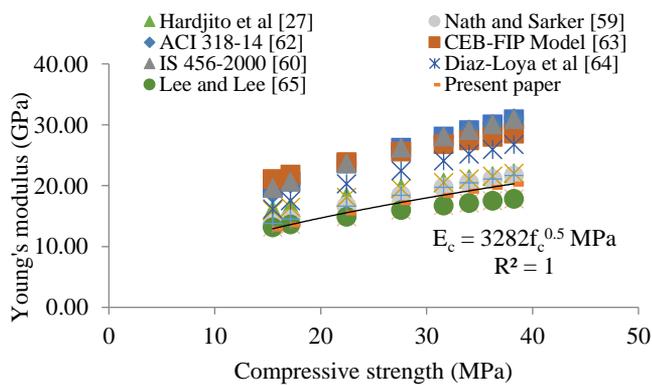


Figure 6. Comparison of predicted young's modulus

5. CONCLUSIONS

The data acquired from the experimental work on GC, the following conclusions are drawn:

Compared to all the mixes of Fly ash-GGBS synthesized GC (F₃₀G₇₀) attained higher compressive strength at 28 days of ambient curing i.e., 38.25 MPa for the same mix splitting tensile and flexural strength values are 6.68 MPa and 9.08 MPa, respectively.

A non-linear numerical model was developed to calculate the empirical relationship between Splitting tensile strength (f_t) Vs. Compressive strength (f_c) and Flexural strength (f_{fs}) Vs. Compressive strength (f_c) of geopolymer concrete (GC) at 28 days of ambient curing. Thus, the proposed equations for ' f_t ' Vs. ' f_c ' and ' f_{fs} ' Vs. ' f_c ' are $f_t=0.51f_c^{0.66}$ and $f_{fs}=0.57f_c^{0.74}$, respectively.

Elastic modulus of GC is found to be 23-28 % less than that of OPC concrete with similar compressive strength at 28 days of ambient curing. With an increase of compressive strength modulus of elasticity also increased.

The equations proposed by CEB-FIP model and ACI 318-14 code are overvalued for the elastic modulus of GC. Therefore, the proposed equation $E_c = 3282 \times \sqrt{f_c}$ is used to predict the elastic modulus of fly ash-GGBS based geopolymer concrete in ambient curing condition.

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NOMENCLATURE

GC	Geopolymer concrete
p	Ultimate load (KN)
l	Supported length (mm)
b	Width of the beam specimen (mm)
d	Depth of the beam specimen (mm)
f_t	Splitting tensile strength
f_{ft}	Flexural strength
f_c	Compressive strength